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(Vulnerability)

EMS-98

7

(Qualitative Method)

### **Vulnerability, and Expected Seismic Performance of Buildings in West Bank, Palestine**

**Abstract** there are different factors affecting the over all vulnerability of a structure in addition to its construction type. These factors are generally applicable to all types of structures. To emphasize the necessary data required for assigning the vulnerability classes for Palestinian buildings, seven , represents the almost the main regions in West Bank, were investigated by collecting information based on the site conditions, regularity and configuration structural and architectural elements of buildings, adjacency, edge material conditions ,etc.

For each city, two representative zones or more were selected for the investigation. The collected data and analysis were determined according to European Macroseismic scale 1998 (EMS) and calibrated by using Japanese qualitative method. The results showed that one third of the investigated buildings belong to seismic vulnerability of class A (Many buildings of class A will suffer heavy damage), whereas about 40 percent of the buildings indicate class B (Many buildings of class B will suffer moderate damage).

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[13] [11] [8] [6]

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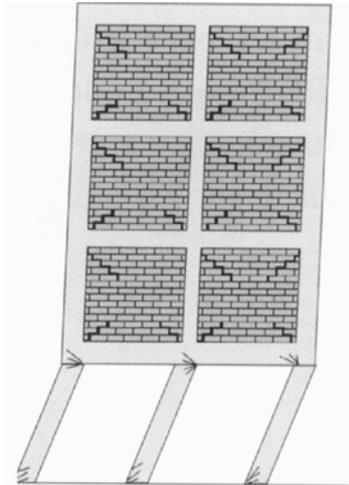
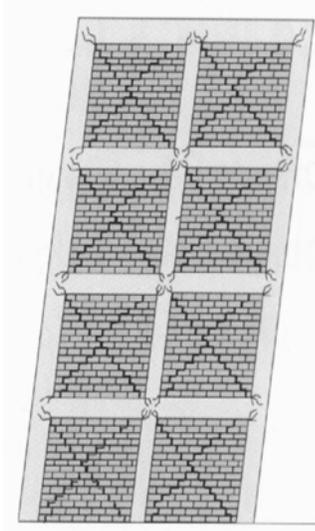
(Soft Story)

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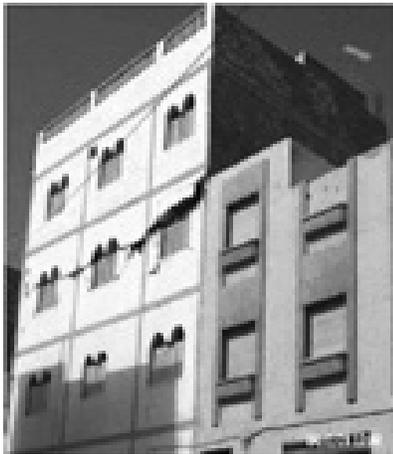
1995



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(Seismic Joints)

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**2004**

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(Cantliever Systems)

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(4)



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(Confinement)

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2003



2003



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**.3**

(Vulnerability)

(Medvedev Sponbeuer MSK (Modified Mercalli) MM  
(Magnitude) M Karink)

"  
.M MM

**1.3**

European Macroseismic )

[7] (Scale

(Masonry Buildings)

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**1.4**

[7]

(Building Types)

[8]

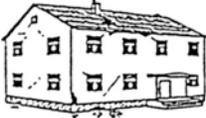
EMS-98

(Vulnerability Classes)

(9 F A ) 6

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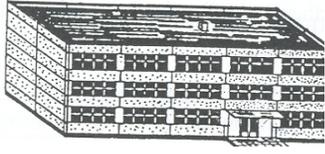
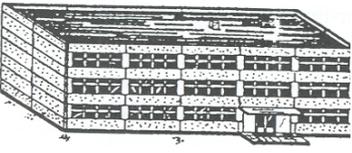
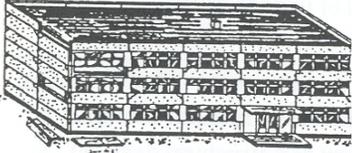
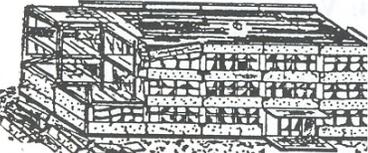
تصنيف الإنهيار في مباني الطوب (المباني غير المسلحة)  
 Classification of damage to masonry building

	(Damage of grade : 1) ) ( )
	(Damage of grade 2 ) : ( )
	(Damage of grade 3) : / ) ( )
	(Damage of grade 4) : ( )
	(Damage of grade 5) : ( )

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(7)

**Classification of damage to buildings of reinforced concrete**

	<p><b>:(Grade1)</b> ) (</p>
	<p><b>:(Grade 2)</b> ( )</p>
	<p><b>:(Grade 3)</b> ) (</p>
	<p><b>:(Grade 4)</b> ) (</p>
	<p><b>:(Grade 5)</b> ( )</p>

**EMS-98**

**:(8)**

فئات قابلية الإصابة						النظام الإنشائي	نوع المبنى
Vulnerability Class							
A	B	C	D	E	F		
○						مباني من الحجارة (دبش قطع غير مصقولة) Rubble stone, Fieldstone	مباني من الطوب (masonry)
○—						مباني طينية ( من اللبن ) adobe (earth brick)	
— ○						مباني من الحجارة البسيطة (أشكالها غير معقدة) simple stone	
	— ○—					مباني من الحجارة الكبيرة قوية متماسكة massive stone	
— ○—						مباني غير مسلحة (حجارة مصنعة ) unreinforced, with manufactured stone units.	
	— ○—					مباني غير مسلحة ( لكن البلاطات مسلحة ) unreinforced, with RC floors	
		— ○—				مباني من الطوب المسلح reinforced or confined	
— — ○—						إطارات غير مصممة لمقاومة الزلازل frame without ERD	مباني من الخرسانة المسلحة (Reinforced Concrete RC)
	— — ○—					إطارات مصممة بتصميم متوسط لمقاومة الزلازل frame with moderate level of ERD	
		— — ○—				إطارات مصممة بتصميم جيد لمقاومة الزلازل frame with high level of ERD	
— ○—						جدران مسلحة غير مصممة لمقاومة الزلازل walls without ERD	
	— ○—					جدران مسلحة مصممة بتصميم متوسط لمقاومة الزلازل walls with moderate level of ERD	
		— ○—				جدران مسلحة مصممة بتصميم جيد لمقاومة الزلازل walls with high level of ERD	
	— — ○—					steel structures منشآت معدنية	
— — ○—						timber structures منشآت خشبية	Wood

○ تشير إلى فئة قابلية الإصابة التي يقع فيها المبنى  
— احتمال انتقال المبنى إلى الفئة الأخرى

ERD : التصميم المقاوم للزلازل (Earthquake Resistant Design)

EMS98

(9)

3.3

(Seismic Intensity)

EMS- 98

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**3.4**

(Damage of grade4) 4

A

.5

(Damage of grade3) 3

B

.4

(Damage of grade 2)2

C

.3

(Damage of grade 2) 2

D

**3.5**

(Damage of grade 5) 5

A

4

B

.5

3

C

.4

2

D

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E

%20-10 % 60-20 %100-60: EMS-98

12 EMS-98  
(MM)

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3.2

(Vulnerability Classes)

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[14] [13] [12] [11] [10] [9] [8] [7] [6]  
( Regular Buildings)

:

.B

( Reinforced concrete frame buildings)

C (Braced buildings)

) D  
( Shear walls  
(9 )

(rigidity)  
( )

A B C

(2 ) (Stiffness)

/ :  
(Variation in Stiffness)

B

) (2

(H) : (1) (L) (M)

(1)

EMS-98

(2)

C

B A

"

D

E F

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فئة قابلية الإصابة				معامل الأهمية (I)	تشكيل المخطل الرئيسي	وجود أنظمة الطيران	وجود أعداد قصيرة	وجود طابق رخو	فواصل زلزالية	عدم تماثل عمودي	عدم تماثل أفقي	نسبة النخافة	حالة المبنى (المواد)	نوع التربة	إحداد الموقع	التصميم الزلزالي	نوع البناء	عوامل	رمز المبنى	الرقم
A	B	C	D																	
				1.2	unsafe	H-W <sub>H</sub>	M	H	-	M	L	8	G	S <sub>C</sub>	M	Without	R.C-Mas	Nablu/Z213	٣٠١	
				1	Safe	L-W <sub>L</sub>	L	-	-	L	-	△3	E	S <sub>B</sub>	-	Without	R.C-Mas	Nablu/Z214	٣٠٢	
				1	unsafe	L-W <sub>L</sub>	M	-	-	M	-	△3	G	S <sub>B</sub>	-	Without	Masonry	Nablu/Z215	٣٠٣	
				1	Safe	M-W <sub>M</sub>	L	L	d=2cm	H	M	△3	B	S <sub>B</sub>	M	Without	Masonry	Nablu/Z216	٣٠٤	
				1	unsafe	-	L	-	d=1cm	M	-	△3	G	S <sub>C</sub>	L	Without	Old Masonry	Nablu/Z217	٣٠٥	
				1	Safe	-	L	-	-	L	-	△3	V.G	S <sub>A</sub>	L	Without	R.C-Mas	Nablu/Z218	٣٠٦	
				1	Safe	L-W <sub>L</sub>	L	L	-	L	M	△3	V.G	S <sub>A</sub>	L	Without	R.C-Mas	Nablu/Z219	٣٠٧	
				1	unsafe	-	L	L	-	L	M	5	G	S <sub>A</sub>	L	Without	R.C-Mas	Nablu/Z220	٣٠٨	
				1	Safe	H-W <sub>H</sub>	L	L	d=2cm	L	H	△3	G	S <sub>B</sub>	M	Without	R.C-Mas	Nablu/Z221	٣٠٩	
				1	Safe	M-W <sub>L</sub>	-	-	-	L	L	△3	V.G	S <sub>B</sub>	L	Without	Masonry	Nablu/Z222	٣١٠	
				1	Safe	M-W <sub>L</sub>	L	-	-	-	-	△3	G	S <sub>B</sub>	L	Without	Masonry	Nablu/Z223	٣١١	
				1	Safe	-	L	-	-	L	L	7	G	S <sub>B</sub>	M	Without	R.C-Mas	Nablu/Z224	٣١٢	
				1	Safe	-	-	-	-	-	-	△3	V.G	S <sub>B</sub>	L	Without	R.C-Mas	Nablu/Z225	٣١٣	
				1	Safe	M-W <sub>H</sub>	M	M	d=1cm	M	M	△3	V.G	S <sub>B</sub>	M	Without	Masonry	Nablu/Z226	٣١٤	
				1	Safe	-	-	-	-	-	-	△3	V.G	S <sub>B</sub>	-	Without	Old Masonry	Nablu/Z227	٣١٥	

L: Low

M: Moderate

H: High

W<sub>L</sub>: Low weight

W<sub>M</sub>: Moderate weight

W<sub>H</sub>: Heavy weight

ERD: Earthquake Resistance Design

E: Excellent

V.G: Very Good

G: Good

B: Bad

V.B: Very Bad

I= 1, Normal, Residential Buildings.

I=1.2, Hazardous Buildings, Schools, Hospitals.

I= 1.5, Essential Buildings, Power- Generating stations, All structures with occupancy grater than 500 Persons.

(-): Not applied or no effect for the mentioned factor.

S<sub>A</sub>: Hard Rock.

S<sub>B</sub>: Rock.

S<sub>C</sub>: Very dense soil and soft rock.

R.C.Mas: Reinforced concrete beams and columns with exterior decorative masonry walls.

(2)

	A	B	C	D	
820	39%	41%	17%	3%	
120	23%	39%	22%	7%	
120	43%	31%	26%	0%	
100	45%	43%	12%	0%	
100	34%	45%	21%	0%	
80	41%	37%	19%	3%	
100	42%	39%	19%	0%	

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.(3)

(3)

9 :			8 :			7 :			
3	4	5	3	4	5	3	4	5	
15 %	23%	15.6%	18%	21%	5.9%				
-	-	-	19%	19%	4.8%	-	-	-	
-	-	-	17%	22%	6.5%	-	-	-	
-	-	-	19%	24%	6.75%	-	-	-	
-	-	-	21%	20%	5.1%	20%	5.1%	-	
-	-	-	18%	22%	6.15%	22%	6.15%	-	
-	-	-	19%	23%	6.3%	-	-	-	

9

$$I_s = E_T \cdot \left( \frac{20}{\dots} \right) \quad (1)$$

$I_s > E_T$  :  
(Capacity > demand)

:

$$I_s = E_o \cdot G \cdot S_o \cdot T \quad (1)$$

$$E_o = \Phi \cdot C \cdot F \quad (2)$$

:

- $E_o$  : the basic seismic index
- $G$  : the geological index
- $S_o$  : the structural design index
- $T$  : the time index
- $\Phi$  : the story index
- $C$  : the strength index
- $F$  : the ductility index

G

$$.1 \quad G$$

$$I_s = \dots \quad (10)$$

.[15]

$E_o$

This seismic protection index  $E_T$  is a one level index and can be Estimated the following equation:

$$E_T = E_S G_G C \quad \dots\dots\dots 1.3$$

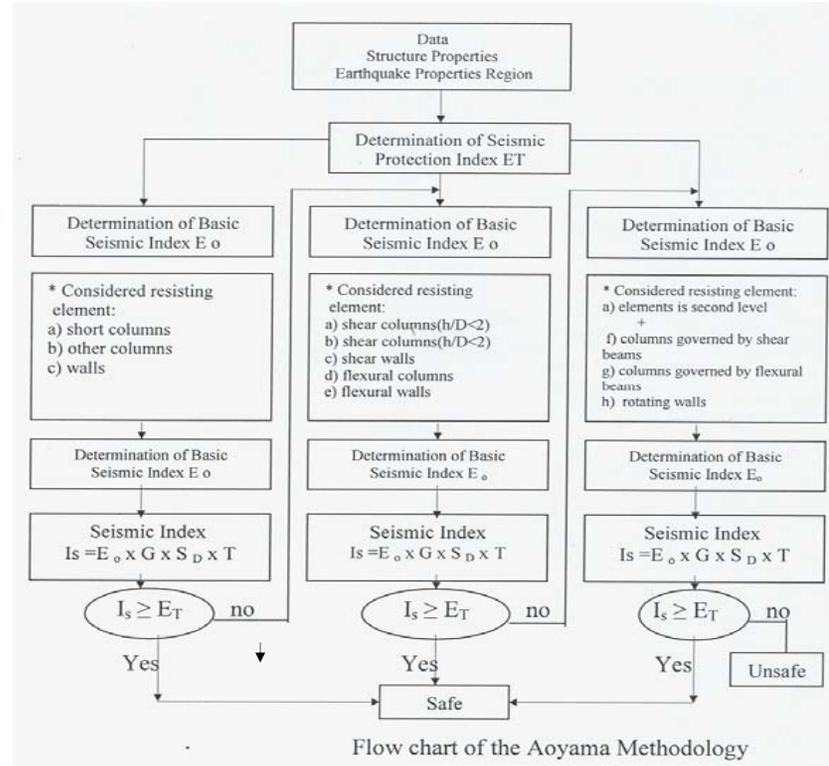
$$E_S = (C_R F) \cdot (a_g \quad \dots\dots\dots 1.4$$

$$C_R \cdot F = 3.15 \sqrt{\frac{T_g}{2T}} \leq 3.15 \quad \text{for flexural yielding type buildings}$$

$$C_R \cdot F = 2.90 \sqrt{\frac{T_g}{2T}} \leq 3.15 \quad \text{for flexural yielding type buildings}$$

Where

- $E_S$  : the basic seismic protection index
- $G_G$  : correction factor for topography
- $C_I$  : importance factor
- $C_R$  : strength ratio (strength divided by the mass and ground peak acceleration)
- $F$  : the ductility index
- $a_g/g$  : the ground peak acceleration divided by gravity acceleration
- $T_g$  : predominant period of the ground
- $T$  : natural period of the building



$$E_T = E_s G_G C_1 \quad (3)$$

$$E_s = (C_{RF}) \cdot (a_g / g) \quad (4)$$

$$E_T = \frac{GG}{(2)} \cdot \frac{CR}{(1)} \cdot \frac{F}{EMS-98} \cdot \frac{E_s}{I} \quad [15]$$

$$[2] \quad 0.20 \quad 0.15 \quad a_g/g$$

$$(0.25 - 0.2) \quad (4) \quad )$$

B A

$$(4) \quad ) C$$

$$)B9 \quad B7 \quad B6 \quad B3 \quad B2 \quad B1$$

$$B8 \quad B5 \quad ($$

$$(9) \quad )$$

$$B3 \quad B8 \quad B5$$

C

$$(10) \quad (4)$$

(T)

(4)

Build	G	S <sub>D</sub>	T <sub>T</sub>	F	C	Q	I <sub>s</sub>
B1	0.8	0.7	0.7	0.45	1.2	1	0.21
B2	0.67	0.8	0.7	0.45	1.25	1	0.21
B3	1	0.9	0.7	0.45	1	1	0.28
B4	0.8	0.6	0.6	0.5	1.5	1.25	0.27
B5	0.8	0.7	0.7	0.55	1.25	1.25	0.34
B6	1	0.6	0.5	0.5	1.5	1.25	0.28
B7	0.9	0.8	0.8	0.55	1.25	1	0.40
B8	1	0.7	0.9	0.55	1.2	1	0.42
B9	0.9	0.7	0.8	0.55	1.35	1	0.37

Build	G <sub>G</sub>	C <sub>I</sub>	T <sub>T</sub>	T	A <sub>g</sub> g	E <sub>T</sub>	E <sub>val</sub>	V <sub>ul</sub>
B1	1	1	0.5	0.2	0.12	0.38	No	A
B2	1	1	0.6	0.3	0.12	0.38	No	A
B3	1	1	0.3	0.25	0.12	0.29	Y	C
B4	1.2	1	0.3	0.4	0.24	0.51	No	A
B5	1.3	1	0.3	0.9	0.24	0.40	No*	B
B6	1.2	1	0.3	0.5	0.24	0.46	No	A
B7	1.1	1	0.4	0.5	0.24	0.53	No	B
B8	1	1	0.4	0.45	0.24	0.436	No*	B
B9	1.1	1	0.35	0.35	0.24	0.54	No	A

(Amr, 1998 and Mario 1994)

:E<sub>va</sub>

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:No\*

.(10)

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