





Support Action for Strengthening PAlestine capabilities for seismic Risk Mitigation SASPARM 2.0

2014 PROJECT FOR CIVIL PROTECTION FINANCIAL INSTRUMENT PREPAREDNESS AND PREVENTION SCHEME

RETROFIT MEASURES PRACTITIONERS (deficiencies)

Pavia – Nablus May 25, 2016



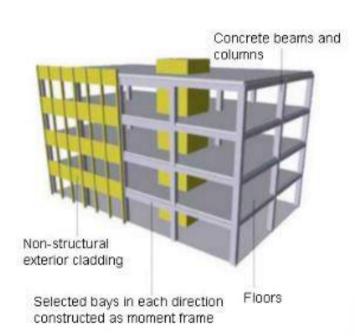
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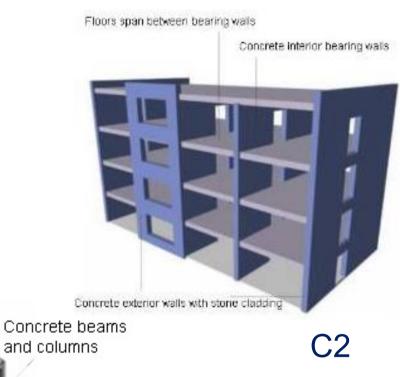
RC TYPOLOGIES



C1

Adapted from FEMA 547: Federal Emergency Management Agency, *Techniques for the seismic rehabilitation of existing buildings*, October 2006.

Missing infill walls



C₁a

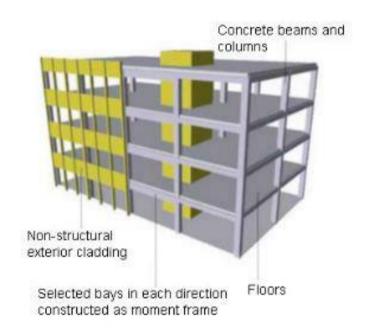
r strengthening PAlestine capabilities for seismic Risk Mitigation : co-funded by ECHO - Humanitarian Aid and Civil Protection







RC FRAME TYPOLOGY



Type C1 can be generically defined as a complete RC frame system of beams and columns supporting slabs.

C1

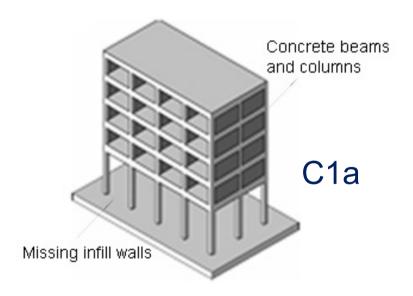






RC FRAME (soft storey) TYPOLOGY

RC buildings with soft storey (C1a) have the same structural characteristics with type C1, but with the peculiarity of lacking significant infill walls in a floor or in a side of it.









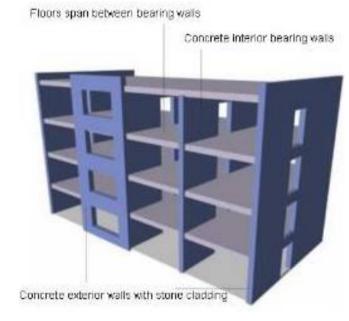


DUAL SYSTEM TYPOLOGY

Building Type C2 is made of RC shear walls.

RC columns may be present for carrying gravity loads only.

It is the only typology that fulfills requirements of Seismic Code.



C2



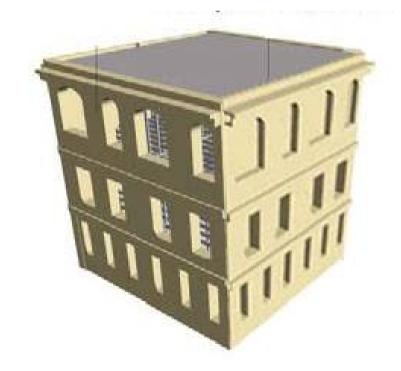




UNREINFORCED MASONRY TYPOLOGY

Building Type URM consists of unreinforced masonry bearing walls.

In general, all walls act as both bearing and shear walls and the floors are concrete slabs cast-in-place.



URM







Category of Seismic Deficiency vs Classes of Rehabilitation Techniques

- ✓ Global Strength / Stiffness;
- ✓ Configuration;
- √ Sectional Detailing;
- ✓ Diaphragms;
- ✓ Foundations.

- ✓ Add elements: to increase strength/stiffness;
- ✓ Enhance performance of existing elements: to increase strength or deformation capacity;
- ✓ Reduce demand: to provide acceptable performance for weak lateral system;
- ✓ Remove selected components: to enhance deformation capacity.









Global	Building Category			
Strength/Stiffness and Configuration	C1/C1a	C2	URM	
Insufficient n° of frames	X			
Short – column mechanism	X			
Infill walls failing or causing torsion	X			
Insufficient in-plane wall strength			X	
Re-entrant corner	X	X	X	
Torsional layout (RC elevator core and staircases)	X	X	X	
Discontinuous walls		X		
Soft-storey	X			



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	Building Category			
Structural Detailing	C1/C1a	C2	URM	
Weak column – strong beam	X			
Inadequate shear strength in column or beam	X	X		
Splices	X			
Insufficient in-plane wall shear strength (web or boundary element)		X		
Insufficient flexural capacity (chord rotation)	X	X		
Brittle failure of coupling beams		X		
Wall inadequate for out-of-plane bending			X	
Unbraced parapet			X	
Poorly anchored veneer or appendages			X	



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Foundations		Building Category		
		C1/C1a	C2	URM
New Foundations	Add shallow found next to existing shallow ones	X	X	X
	Add deep foundations next to existing shallow ones	X	X	X
Existing Shallow Foundations	Add Micropiles			
	Enlarge exisisting spread footing	X	X	X
Existing Deep Foundations	Add a Mat Foundation, Extended Pile Cap or Grade Beam	X	X	X
Ground Improvement	Compaction Grouting	X	X	X
	Permeation Grouting	X	X	X



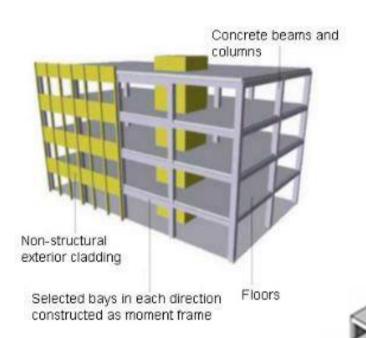






RC MAIN DEFICIENCIES

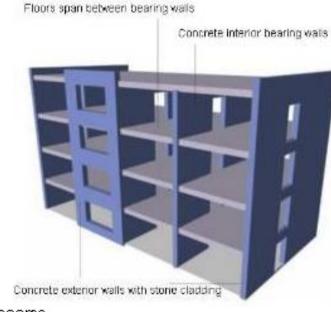
FRAME and DUAL SYSTEM TYPOLOGY



C1

Adapted from FEMA 547: Federal Emergency Management Agency, *Techniques for the seismic rehabilitation of existing buildings*, October 2006.

Missing infill walls



Concrete beams and columns

C2

C₁a

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RC Deficiencies – Global Strength /Stiffness

✓ Insufficient number of frames



- Frames in only one direction;
- Frame not related to each other in order to resist in both direction;
- Frame would present irregularities.

Italian Department of Civil Protection, Manuale per la compilazione della scheda di 1º livello di rilevamento danno, pronto intervento e agibilità per edifici ordinari nell'emergenza post-sisma (AeDES), 2014.



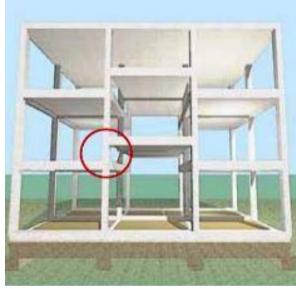




RC Deficiencies – Global Strength /Stiffness

√ Frames with inadequate stiffness







Italian Department of Civil Protection, Manuale per la compilazione della scheda di 1° livello di rilevamento danno, pronto intervento e agibilità per edifici ordinari nell'emergenza post-sisma (AeDES), 2014.

- Tall column with instability problem;
- Short column with high concentration of shear stress.









RC Deficiencies – Global Strength /Stiffness

✓ Infill walls failing or causing torsion







- Infill walls collapse out-of-plane due to inadequate fixing to the frame (causing only mass addition);
- Infill walls not appropriate arranged to the frame structure cause asymmetrical behavior.

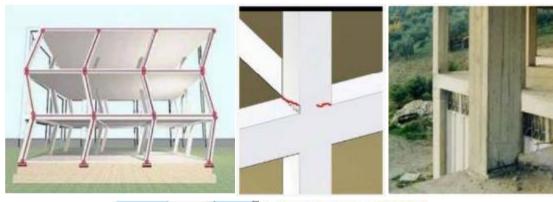


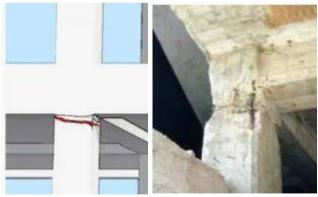






✓ Weak column – strong beam (should be avoided)





- Column sway plastic mechanism
- Plastic hinge at the top or the base of the column due to an element with higher stiffness

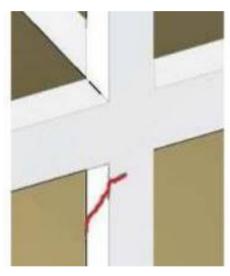




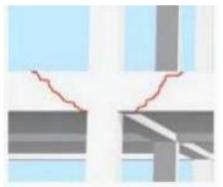




✓ Inadequate transverse reinforcement









- Inadequate shear resistance in beam and/or columns;
- Poor restraints
 against buckling of
 longitudinal
 reinforcement.
- Low member ductility

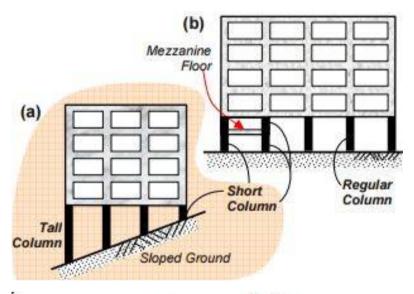
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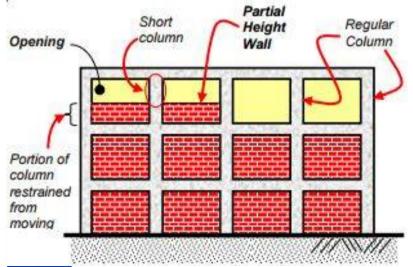






✓ Short column mechanism





- Building is rested on sloped ground;
- In columns supporting mezzanine floors or loft slabs that are added in between two regular floors;
- Walls of partial height built, adjacent columns behave as short columns due to presence of these walls.

C.V.R. Murty, *Why are Short Columns More Damaged during Earthquakes?*, Indian Institute of Technology, Kanpur, 2004.

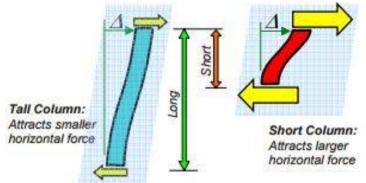
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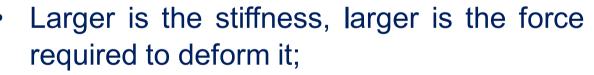


✓ Short column mechanism



Poor behavior of short columns because a tall column and a short column of same cross-section move horizontally by same amount Δ ;





- If a short column not adequately designed for a large force, it can suffer significant damage;
- X-shaped cracking due to shear failure.











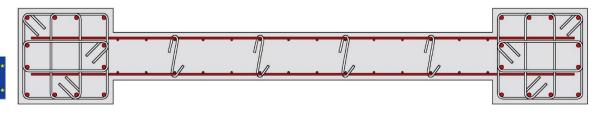
RC Deficiencies – Sectional Detailing

✓ Insufficient wall shear strength (shear wall typology)



Desirable configuration

- Inadequate transverse reinforcement in web (horiz. rebars);
- Inadequate boundary elements (stirrups);
- Early brittle failure of crucial elements.

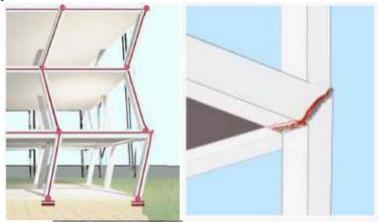








√ Splices





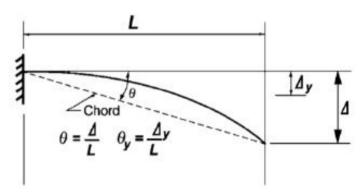
- Beam-column joint with inadequate reinforcement;
- Beam-column joint with improper anchorage of longitudinal beam reinforcement.



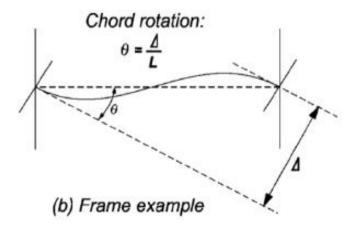




✓ Insufficient flexural capacity



(a) Cantilever example



- Inadequate confinement of beams/columns/walls in the critical regions;
- Early plastic hinge formation;
- Sections not reach large curvature values limiting the chord rotation capacity (ductility) and energy dissipation of the elements.

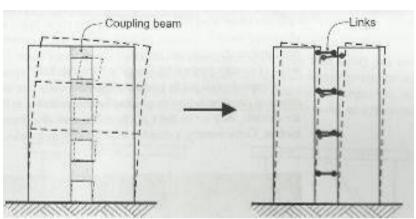








✓ Brittle failure of coupling beams





- The behavior of coupled shear walls is governed by coupling beams;
- Purpose of beams between coupled walls is transfer shear from one wall to the other;
- Many coupling beams are designed as conventional flexural members;
- Beams will inevitably fail in diagonal tension;
- Diagonal failure crack will divide a beam into two triangular parts.

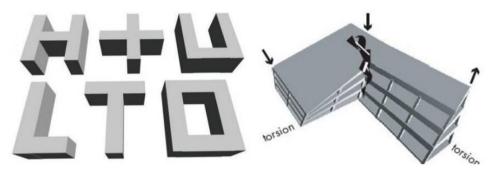




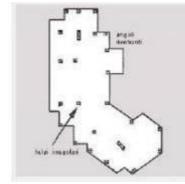




✓ Re-entrant corner



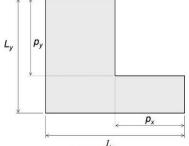
FEMA 451, NEHRP Recommended Provisions: Design Examples, August 2006.





- H, I, T, L, C, U shapes;
- Causes torsion due to center of mass ≠ center of rigidity
- Produces differential motion between different wings of the building leading to local stress concentration.





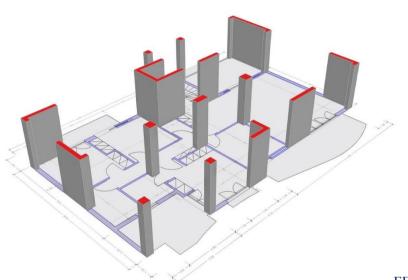
M. Divyashree, G. Sippada, *Seismic Behavior of RC Building with Re-entrant Corners and Strengthening*, IOSR Journal of Mechanical and Civil Engineering, e-ISSN: 2278-1684, pp. 69-69, 2013.

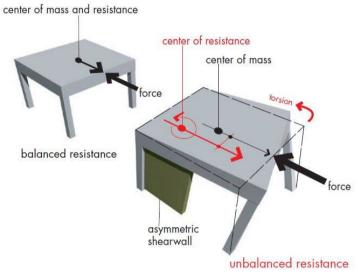






✓ Torsional Layout (RC elevator and staircase) frame typology





FEMA 451, NEHRP Recommended Provisions: Design Examples, August 2006.

- Lack of balance between the location of the resisting elements and the arrangement of the building mass;
- Eccentricity problem: center of mass ≠ center of resistance;
- Eccentricity during earthquake leads building rotating around its center of resistance, creating torsion in plan.

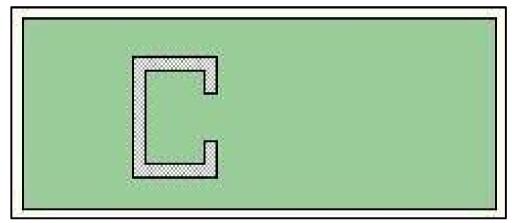








✓ Torsional Layout (RC elevator and staircase) shear wall typology



- Shear walls are concentrated around elevator shafts and staircases;
- Lack of good torsional response due to stiffness eccentricity.

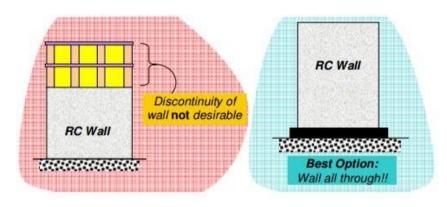


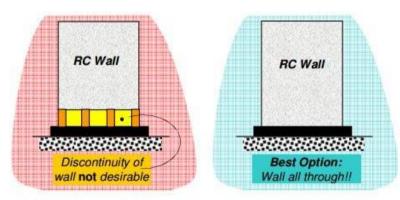






✓ Discontinuous walls (shear wall typology)





- Serious overstressing due to the non-continuity of the load path through the walls from the roof to foundation;
- Building unable to resist seismic forces regardless of the strength of the existing elements;
- Caution for gaps in the load path (e.g. discontinuity in height).





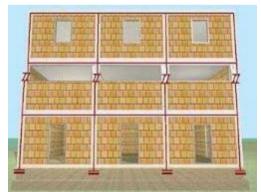




✓ Soft storey (frame typology)









- Abrupt variation in storey stiffness along vertical direction;
- A storey has lower stiffness respect to upper storeys triggering soft storey mechanism failure;
 - Plastic hinges form at column ends of one floor;
 - Leads to column sway plastic mechanism and to low energy dissipation

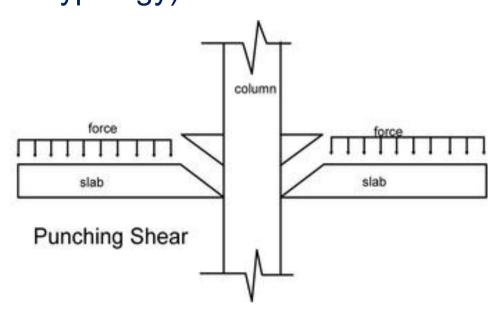






RC Deficiencies - Diaphragms

✓ Punching shear failure of slab-column connection (frame typology)



- Due to high localized forces in RC slabs;
- In flat slab structures this occurs at column support points;
- Catastrophic failure because no visible signs are shown prior to failure.



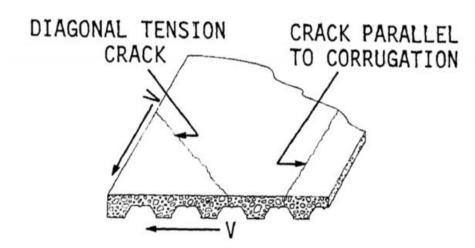






RC Deficiencies - Diaphragms

✓ Inadequate in-plane shear capacity



- When concrete stress reaches its tensile limit, diagonal cracks (at an angle of approximately 45°) occur across the slab;
- Direct shearing of the concrete along a line parallel to the deck corrugations.









RC Deficiencies - Diaphragms

✓ Excessive stresses at openings and irregularities



M. Mota, M. Kamara, *Floor Openings in Two-Way Slab*, Concrete International, American Concrete Institute, Farmington Hills, USA, July 2006, pp. 33-36.

- Openings in existing slabs should be approached with caution and avoided if possible;
- Effect on the structural integrity of the slab must be analysed: especially for excess capacity and possible moment redistribution.









URM MAIN DEFICIENCIES UNREINFORCED MASONRY TYPOLOGY



URM

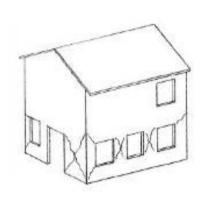




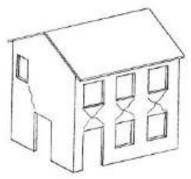


URM Deficiencies – Global Strength / Stiffness

✓ Insufficient in-plane wall strength









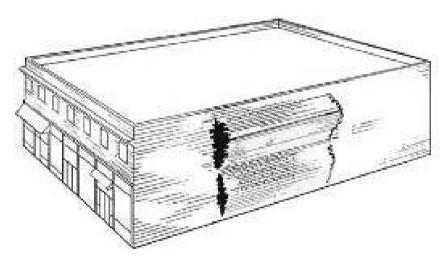
- Unreinforced walls incapable of withstanding severe repeated load reversals;
- Severe strength degradation characteristics are double-diagonal (X) shear cracking.







✓ Wall inadequacy for out-of-plane bending



FEMA 774: Federal Emergency Management Agency, *Unreinforced Masonry Buildings and Earthquake*, October 2009.

 The unbraced length of brittle walls causes them to buckle out-of-plane during lateral loading.

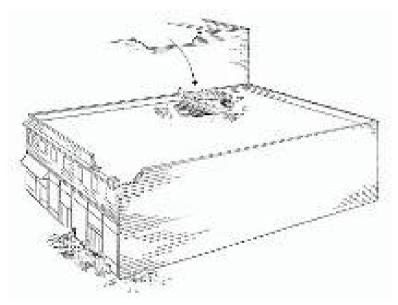








✓ Unbraced parapet



FEMA 774: Federal Emergency Management Agency, *Unreinforced Masonry Buildings and Earthquake*, October 2009.

 Parapet failures cause damages not only to the building itself, but to nearby buildings.

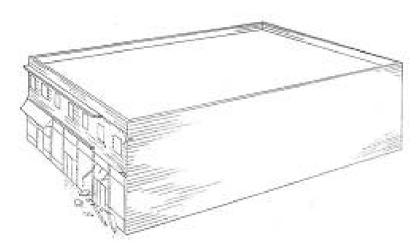








✓ Poorly anchored veneer or appendages



FEMA 774: Federal Emergency Management Agency, *Unreinforced Masonry Buildings and Earthquake*, October 2009.

 Non-structural elements can cause damages also in surroundings area.

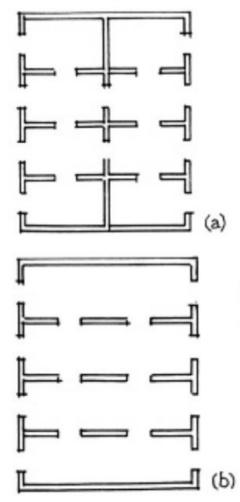








✓ Insufficient in-plane wall strength



 Symmetrical buildings of simple form perform better than complex shapes where walls are asymmetrically distributed on plan.







URM Deficiencies – Diaphragms

✓ Inadequate in-plane strength and stiffness



2005 Pakistan Earthquake

- When concrete stress reaches its tensile limit, diagonal cracks (at an angle of approximately 45°) occur across the slab;
- Direct shearing of the concrete along a line parallel to the deck corrugations.









URM Deficiencies – Diaphragms

✓ Inadequate chord capacity





 Walls must be tied to the horizontal diaphragms (roof and floors) to increase their resiliency to out-of-plane loading.

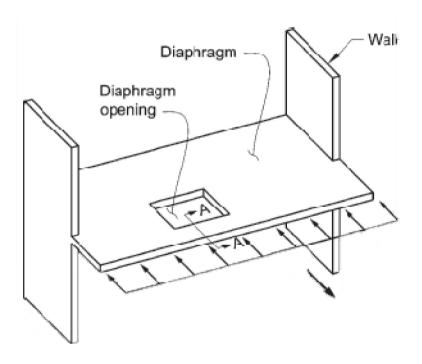






URM Deficiencies – Diaphragms

✓ Excessive stresses at openings and irregularities



- Large openings limit strength of diaphragm to transfer lateral forces;
- Creation of a vulnerable structural condition related to irregular construction (irregular stiffness);
- Opening/absence of a complete floor diaphragm at one or more levels results in inability to transfer lateral forces from walls at one level to walls and floors at adjacent levels.









The retrofitting measures follow ...

