





Support Action for Strengthening PAlestine capabilities for seismic Risk Mitigation SASPARM 2.0

2014 PROJECT FOR CIVIL PROTECTION FINANCIAL INSTRUMENT PREPAREDNESS AND PREVENTION SCHEME

RETROFIT MEASURES PRACTITIONERS (introduction)

Pavia – Nablus May 25, 2016









Presentation outline

- General overview of structural analysis on existing buildings and concept of capacity design;
- 2. Classes of rehabilitation techniques;
- 3. RC: main deficiencies;
- 4. URM: main deficiencies;
- 5. Retrofit measures: common and advanced ones;
- 6. Implementation in Palestine.



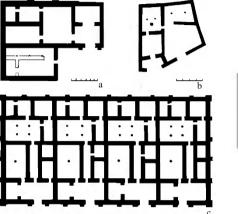






Retrofitting of existing structures

1. Gather information about existing structure (plans)





2. Gather information about material conditions (non destructive testing

methods)



- 3. Structural assessment of existing structure
- Propose retrofitting measures and assess corresponding direct and indirect cost



5. Implementation of retrofitting measures



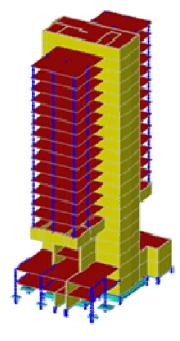






Structural assessment of existing structure

- 1. No specific code for Palestine, code of reference UBC/JBC.
- 2. More complex than designing a new one. Less flexibility and more unknowns.
- 3. Need for inelastic nonlinear analysis.
- 4. Empirical approaches cannot be used to non damaged structures





SASPARM 2.0

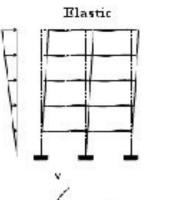


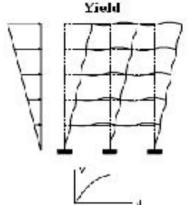


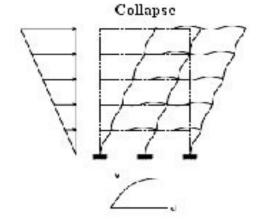


Structural analysis

- 1. Only 3D models.
- 2. Impossible to do by hand.







- 3. Only inelastic nonlinear analysis valid to get reasonable cost-benefit estimates.
- 4. If the modeling assumptions are very conservative (linear elastic analysis), the "replacement cost new" can be quite close to the retrofitting one.
- 5. Pushover inelastic static analysis can be used for regular structures without dominant torsional modes.
- 6. Time history analysis is much more complex and computationally heavy. Record selection process is also very demanding.









- ✓ The modern way of designing new structures or redesigning existing ones;
- ✓ Capacity design: designing flexural capacities of sections of a structure based on its behavior in responding to seismic actions;
- ✓ Assumption: critical regions occur simultaneously at predetermined locations to form a collapse mechanism simulating ductile behavior.









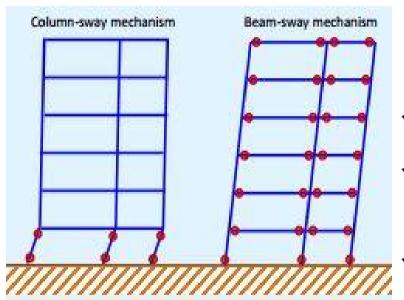
- ✓ Key feature: avoid undesirable modes of failure (shear).
- ✓ Capacity design: aims at establishing a favorable hierarchy of strength in the structures;
- ✓ Strength of columns is higher than that of adjacent beams, with possible allowance for beam over strength weak beam strong column concept;











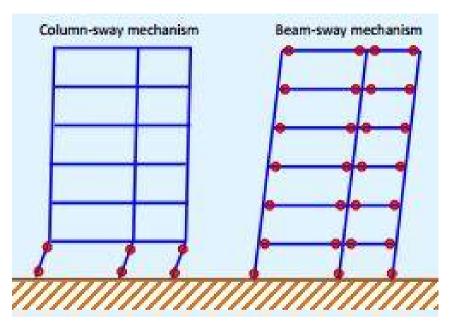
- ✓ Even if weak beam strong column concept is applied, a column plastic hinges must still form at the base of the column;
- ✓ Beams yield before columns;
- ✓ Column sway mechanism is avoided in the structure;
- ✓ Larger energy dissipation and drift capacity











- ✓ Better to develop plastic hinges in all beams instead of only in the first storey column;
- ✓ Overall ductility demand (large deflections) is much more readily achieved;
- ✓ The column hinge mechanism imposes plastic hinge rotations;
- ✓ Good detailing. Small spacing and large diameter of stirrups (i.e. Φ10/100 in mm)









End of introduction.

Part 3 follows....

